PROPOSED SITE SERVICING STRATEGY

RESIDENTIAL TOWNHOUSE DEVELOPMENT 634 COMMISSIONERS ROAD WEST

APRIL, 2023

Revised – OCTOBER 2023, New Site Plan

Ref. No. – 22.164

Prepared for: Royal Premier Homes 509 Commissioners Rd W #425 London, ON N6K 1J5

Prepared by:

ENG PLUS Eng Plus Ltd. 100-609 William Street London, ON N6B 3G1

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1.0 INTRODUCTION

Eng Plus Ltd. has been retained by Royal Premier Homes to prepare a Servicing and Stormwater Management Report to support a Site Plan Application for the proposed infill residential townhouse development located at 634 Commissioners Road West in the City of London.

Figure 1 Key Plan shows the site location, Appendix C.

2.0 EXISTING CONDITIONS

The site is located at 634 Commissioners Road W. on the south side of Commissioners Road West and approximately 475 metres west of Wonderland Road South. The site frontage is 88.0 meters and total site area is 0.445 hectares.

The site currently houses an existing (c. 1850) Georgian-Style Dwelling that is listed on the City's Register of Cultural Heritage Resources. There are existing residential lands uses adjoining the site. The subject lands have been re-zoned to Residential R5-7(30).

The existing topography of the subject property is split in approximately middle of the site and slopes northeast and northwest towards Commissioners Road West to the ditches along the road.

3.0 PROPOSED DEVELOPMENT

The site plan has been changed from the original development proposal.

The proposal now is for two stacked back-to-back townhouse buildings containing a total of 28 new townhouse units and the retention of the existing c.1850 Georgian Style dwelling as a single detached unit.

The revised preliminary concept plan is shown in **Appendix D**.

4.0 WATER SERVICING

There are existing 900mm diameter watermain and 300mm diameter high-level watermain on Commissioners Road W. Water service for the development will be provided by connecting to the existing 300mm diameter high-level watermain.

Watermains on-site were sized based on EPANET simulations for the Average Day and Maximum Hour demand scenarios. The high-level boundary condition (335m) for Westmount Area was used in the model in accordance with City requirements. The proposed water service mains for the site will include 50mm diameter Municipex watermains. Each unit will be individually serviced by a 25mm diameter PEX water service.

The following domestic water demands have been calculated for the proposed stacked townhouse buildings:

Average Day Demand for 8-units = 3.4 L/min; Maximum Hour Demand = 26.5 L/min. Average Day Demand for 20-units (plus ex. House) = 9.0 L/min; Maximum Hour Demand = 70.4 L/min.



The above demands are based on the design criteria outlined in the City of London's Design Specifications and Requirements Manual, including a boundary condition of 335m, population density of 2.4 people per unit, and an average domestic flow rate of 255 L/cap/day. Refer to **Appendix A** for calculations. The following table summarizes the results of the EPANET model.

Table 1. Summary of EPANET Modelling Results

Node	Operating Condition	Pressure (Min. 275 kPa or 28m head)	Velocity (m/s)	Water Quality (hours)
8-Units	Average Day Demand	517 kPa (52.8m head)	0.03	0.34
	Peak Hour Demand	510 kPa (51.95m head)	0.23	-
20-Units	Average Day Demand	531 kPa (54.17m head)	0.08	0.22
(+ ex.)	Peak Hour Demand	516 kPa (52.65m head)	0.60	-

The modelled results in the above table indicate that the proposed water service for the development will have adequate pressures under the Average Day and Peak Hour Demand scenarios. Further, the velocity in the system is below the maximum 1.5m/s under the Peak Hour scenario. Also, water quality time is well below the maximum limit of 72 hours.

Fire Protection: Since the proposed buildings are Part 9 of the Ontario Building Code, there is no requirement for on-site fire hydrants for firefighting. There are existing hydrants located on the north boulevard of Commissioners Rd. W at Nottinghill Rd and Westmount Cres. Both of the existing hydrants are located more than 90m to the proposed Townhouse development. A new fire hydrant is proposed on Commissioners Rd. in front of the site to meet the 90m distance to the new proposed buildings. The new fire hydrant on Commissioners Road also provide additional fire protection for the existing dwellings in the area. New location is shown on the attached FIG. 4, Servicing Strategy in **Appendix D**.

There is an existing 900mm diameter trunk sewer on Commissioners Road W. It will be required to cross above this trunk watermain when making the new water and sanitary connections for the proposed development. The crossing shall be constructed in accordance with City of London standard 7.4.7.3 including providing 0.6m of clearance above the trunk watermain.

A new 50mm diameter Municipex water service connection to the existing 300mm main is proposed to service the site.

Existing house demand also included in the calculation. Details calculations can be found in **Appendix A**.

5.0 SANITARY SERVICING

There is no municipal sanitary sewer fronting the site. The municipal sanitary sewer is stop just east of the site. This property was included in the design of this sanitary sewer as a single family lot (per City As-built Plan No. 10,589).



It is proposed to develop 28 stacked back-to-back townhouse units and 1 heritage building for total of 29 units (the site area is 0.449 hectares site). With total population of 71 people (2.4 ppu * 28 + 3 persons per unit).

According to City as-built drawing No. 9993, Sanitary Drainage Areas, Rosecliffe Garden Estate, Feb, 1988. The existing 250mm diameter sanitary sewer on Rosecliffe Terrace had been designed with the external area of 4.12 hectares and populations of 273 people, the allowable flows on Rosecliffe Terace is **3.69 l/s**

We have updated the sanitary sewer design with the current design standards and the added 28 townhouse units (2.4 person per unit). As per the updated design sheet attached, the proposed design flow to the existing sewer on Rosecliffe Terrace is **1.76** l/s (total population of 106 people), less than the allowable design flowrate above (**3.69** l/s). Therefor there is no capacity issue with the proposed infill development.

The sanitary service for the site will be connected to the new extension of the existing sanitary sewer on Commissioners Road West from the existing manhole just east of the site.

The new sanitary PDC of 200mm diameter at 1.0% slope is proposed to connect to the existing sanitary manhole on Commissioners Road West, servicing the site.

All the proposed units and the existing dwelling will have PDC connecting to an internal sanitary sewer system. An inspection manhole is proposed onsite before the connection to the municipal sewers on Commissioners Road West. A schematic of the proposed service is attached.

6.0 STORMWATER SERVICING STRATEGY

6.1 Existing Site Drainage and Approved Drainage Plan

Currently, the site drainage is high along the back and the stormwater is generally draining north, northeast to the existing swales on the property to the east and then to the existing roadside ditches along Commissioners Road West.

As per City of London as-built drawing # 16954 dated Sept. 2001 (attached), the drainage from the site had been accounted for in the existing sewers on Commissioners Road West. The allowable runoff coefficient for the site is C=0.5 outleting to the ditch inlet catchbasin in front of 610 Commissioners Road W. approximately 50 meters east of the site.

The allowable peak flows from the site is 34 l/s and 81 l/s for 2-year and 100-year storm events respectively. Calculations attached.

The approved drainage for the site is shown on Sheet 2, Existing Conditions.



6.2 Post-Development Stormwater Servicing Proposal

As indicated, the site is to intensify with additional 10 townhouse units in 2 building blocks, the existing heritage house is to remain as single detached unit.

The proposed development include surface parking, driveways, and landscape areas.

The actual runoff co-efficient "C" is calculated as per the final site plan as per table below:

Danasiation	Post-Development Runoff Coefficient				
Description	Area (m²)	Runoff Coefficient (C)	CxA		
Asphalt	1088	0.90	980		
Concrete	219	0.90	197		
Building	1012	0.90	911		
Grass	1669	0.25	417		
<u>Total</u>	3988		2505		

Composite C 0.63

As per City of London Design Specifications & Requirements Manual, the runoff coefficient C=0.65 is used for the propose townhouse development.

The increase in the "C" value will result in increase in post-development stormwater runoff from the site. It is proposed to control post-development runoff from the site to the pre-approved flow conditions of C=0.50 so that the downstream storm sewer system will not be affected.

Under the post-development condition, the stormwater runoff from the entire site will be directed north toward Commissioners Road West. Quality and quality control are provided onsite before the runoff leaving the site. The strategies for servicing the site are as per below:

6.2.1 Stormwater Management – Quantity Control

For the post-development conditions, onsite storage is provided so that the total post-development peak flows are not more than the approved flow rates from above.

For minor storm events, 2-year storm, an underground storage system is proposed to store the excess post-development runoff to control the flowrate to the above 2-year controlled flow (34 l/s). There will be no surface ponding during minor storm events.

An orifice is proposed at the outlet to limit the flow from the development area to 2-year storm events and 35 l/s for 100-year storm events.

In the major storm events (up to 100-year storm), extra storage is to be provided on the surface on top of the catchbasin to a maximum depth of 0.3 meters. When the storage capacity of the ponding areas on top of the catchbasins is exceeded, stormwater runoff will overflow and follow the existing overland flow route. Overland flow routes is



provided to safely convey the major storm surface runoff from the site to the existing overland flow route on Commissioners Road W.

Storm Outlet: The outlet from the site is the new proposed 450mm diameter storm PDC connecting to the extension of 450mm diameter storm sewer on the north side of Commissioners Road W.

6.2.2 Quality Control

The proposed development has less than 30 parking spaces, as per the City of London's requirements for on-site private stormwater system, the stormwater runoff from the site must meet the "normal" protection level for water quality control or 70% TSS removal) since there is no downstream quality controls in place.

An oil/grit separator is proposed for treating the runoff water before leaving the site.

7.0 EROSION AND SEDIMENT CONTROL

Sediment control measures are intended to intercept sediment that is suspended in stormwater runoff, prior to reaching the receiving water course. To control sediment transport during construction, the following measures will be implemented and recommended:

- Install silt fence to filter and detain runoff around stockpiles, along grading limits and neighboring property boundaries susceptible to receiving drainage from the subject site;
- Stockpiles to be away from the proposed stormwater storage areas;
- Straw bales and/or riprap check dams in drainage swales and ditches where necessary, to reduce erosive velocities. Check dams should be inspected and maintained during construction (with accumulated sediment removed).

Erosion and sediment control measures should be inspected daily and after every rainfall to determine maintenance, repair or replacement requirements. Sediments or granulars that enter site drainage systems shall be removed immediately by the contractor. These measures will be implemented prior to the commencement of construction and maintained in good order until the site has been paved and vegetation has been established.



8.0 CONCLUSIONS AND RECOMMENDATIONS

- Water servicing for the proposed development is provided by connecting to the existing 300mm diameter watermain on Commissioners Road West;
- The proposed building will be serviced by a new sanitary PDC that will connect to the extension of the existing municipal sanitary sewer system on Commissioners Road West;
- Stormwater quantity and quality impacts from the development will be mitigated through implementation of stormwater control measures including onsite storage and oil / grit separator. The new storm PDC is connecting to the extension of the existing municipal storm sewer system on Commissioners Road West;
- Erosion and sediment control measures will be implemented prior to, during and after construction.

We trust the information presented in this report meets your current requirements. Please do not hesitate to contact us should you have any questions or concerns.





	Residential	Townhouse	Devel	onmen
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APPENDIX A <u>Water Service Calculations</u>

Water Service Calculations

Date: 16-Oct-23

Project: 634 Commissioners Rd. W Stacked Back-to-Back Townhouse Development

Hydraulic Criteria and EPANET Input Data

Average Day Domestic Flow Requirement 255 L/cap/day

Minimum Average Day Demand 275 kPa (40psi, pressure head=28m)
Minimum Max. Peak Hourly Demand 275 kPa (40psi, pressure head=28m)

Maximum Hour Peaking Factor 7.8

Friction Factors

Pipe Diameters	C-Factor
100mm and 150mm	100
200mm and 250mm	110
300mm	120

Maximum Velocity - Max Hour Domestic Flow 1.5 m/s

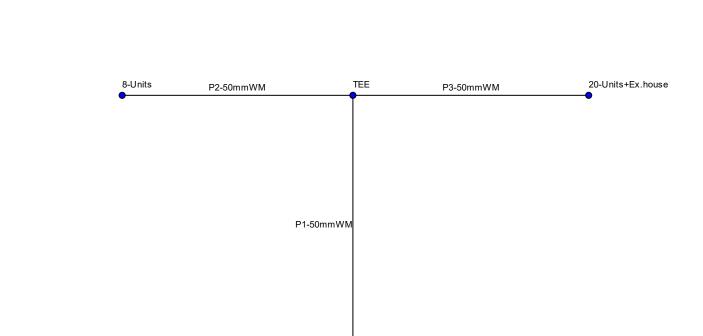
Domestic Water Demand Calculation

Residential Population

Total number of new units is 28 at 2.4 people per unit Existing house at 3 people	67.2 people 3 people		
Average Day Demand - 8 units	3.4	L/min	0.057 L/s
Maximum Hour Demand - 8 units	26.5	L/min	0.442 L/s
Average Day Demand - 20 units + Ex. House	9.0	L/min	0.151 L/s
Maximum Hour Demand - 20 units + Ex. House	70.4	L/min	1.174 L/s

634 Commissioners Rd. W - Proposed Water Service





Ex.300mmWM

Age 1.00 24.00 36.00 72.00 hours

Page 1		2023-10-16 4:09:34 PM
******	*******	******
*	EPANET	*
*	Hydraulic and Water Quality	*
*	Analysis for Pipe Networks	*
*	Version 2.2	*
******	*******	*******

Input File: Water Service_Avg. Day.net

475 Grey Street - Proposed Water Service

Link - Node Table:

Link	Start	End	Length	Diameter	
ID	Node	Node	m	mm	
P2-50mmWM	TEE	8-Units	30	50	
P3-50mmWM	TEE	20-Units+Ex.house	4	5 50	
P1-50mmWM	Ex.300mmWM	TEE	21	50	

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality hours
TEE	0.00	334.98	53.48	0.06
8-Units	0.06	334.98	52.78	0.34
20-Units+Ex.house	0.15	334.97	54.17	0.22
Ex.300mmWM	-0.21	335.00	0.00	0.00 Reservoir

Link Results:

Link ID	Flow Ve	elocityUnit m/s	Headloss m/km	Status
P2-50mmWM	0.06	0.03	0.06	Open
P3-50mmWM	0.15	0.08	0.39	Open
P1-50mmWM	0.21	0.11	0.78	Open

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*	EPANET	*
*	Hydraulic and Water Quality	*
*	Analysis for Pipe Networks	*
*	Version 2.2	*
******	*******	******

Input File: Water Service_Peak hour.net

475 Grey Street - Proposed Water Service

Link - Node Table:

Link	Start	End	Length	Diameter
ID	Node	Node	m	mm
P2-50mmWM	TEE	8-Units	30	50
P3-50mmWM	TEE	20-Units+Ex.house	4	5 50
P1-50mmWM	Ex.300mmWM	TEE	21	50

Node Results:

Node ID	Demand LPS	Head m	Pressure m	Quality hours
TEE	0.00	334.24	52.74	0.01
8-Units	0.44	334.15	51.95	0.04
20-Units+Ex.house	1.18	333.45	52.65	0.03
Ex.300mmWM	-1.62	335.00	0.00	0.00 Reservoir

Link Results:

Link	Flow	VelocityUnit	Headloss	Status
ID	LPS	m/s	m/km	
P2-50mmWM	0.44	0.23	2.89	Open
P3-50mmWM	1.18	0.60	17.50	Open
P1-50mmWM	1.62	0.83	36.20	Open

Residential	Townhouse	Deve	lopment
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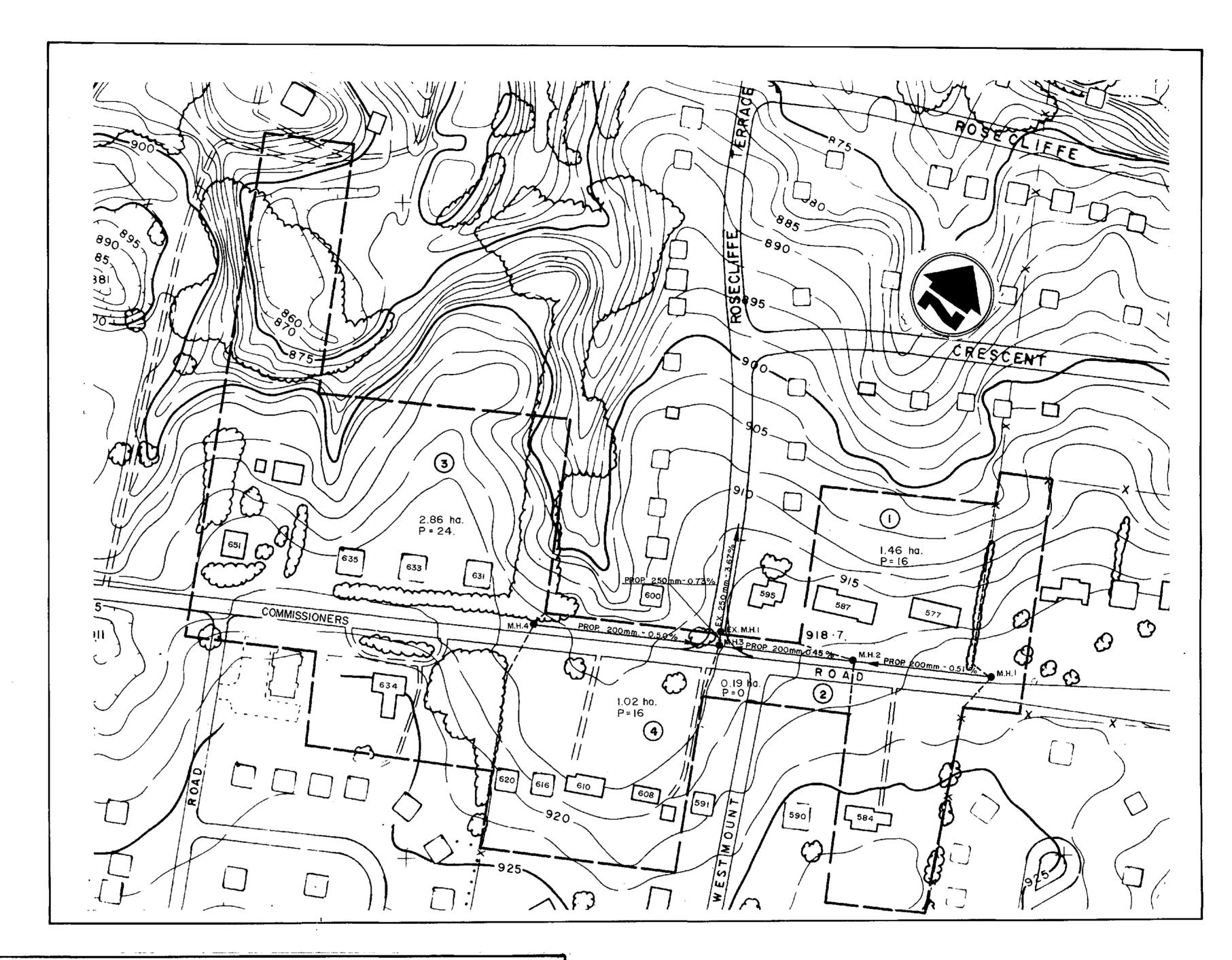
APPENDIX B <u>Updated Sanitary Design Sheet – Sanitary As-Built</u>

RESIDENTIAL POPULATION DE	<u>NSITIES</u>			SANITARY SEWER DESIGN SHEET	DESIGN CRITERIA		
	Lot Basis		Hectare Basis	CITY OF LONDON	PER CAPITA FLOW	=	230 l/cap/d
LOW DENSITY	3	ppu	30 upha		INFILTRATION	=	8640 l/ha/d
MED. DENSITY (TOWNHOUSES)	2.4	ppu	75 upha		PEAKING FACTOR	=	Harmon Formula
HIGH DENSITY	1.6	ppu	150 upha				M= 1 + 14 /(4+P^1/2)

PROJECT NAME: 634 Commissioners Road West - Proposed Townhouse Development

S	SEWER LOCATION	N			AREA		TOTAL	RE	SIDENTIAL	. AREA & F	POPULATION	ON		5	SEWAGE FLC	W			SEWER	R DESIGN	1	
AREA STR	REET F	ROM	TO	NET	GROSS	CUM.	CUM.	PER	PERS.	NO.	Δ	TOTAL	HARMON	INFILT.	SEWAGE	TOTAL	"n"	CALC.	NOM.	PIPE	CAPACITY	VELOCITY
NO.		MH	MH	AREA	AREA	AREA	AREA		PER	OF	POPUL.	CUM.	PEAKING					PIPE D	PIPE D	SLOPE	Q	(0.60 min.)
				ha.	ha.	ha.	ha.	ha.	UNIT	UNITS		POPUL.	FACTOR	l/s	l/s	l/s		mm	mm	%	I/s	m/s
A1 Comm. Rd V	V. E	≣x. 1	Ex. 2	1.460			1.460	60.00	3.0	4	12	12	4.4067	0.15	0.15	0.30	0.013	39.2	200	0.50	23.20	0.74
A2		Ex. 2	Ex. 3	0.19			1.65	60.00	0.0	0	0	12	4.4067	0.17	0.15	0.32	0.013	41.1	200	0.44	21.76	0.69
									28	THs + 1 si	ngle											
A3 Comm. Rd V	N. :	Site	Ex. 4	0.445			0.445		2.4	29	70	70	4.2826	0.04	0.88	0.92	0.013	52.5	200	1.00	32.80	1.04
A3	ı	Fut.	Ex.4	1.30	*		1.75		3.0	4	12	82	4.2659	0.17	1.03	1.20						
A4	E	Ex.4	Ex.3	1.02			2.77	60.00	3.0	4	12	94	4.2506	0.28	1.17	1.45	0.013	71.0	200	0.49	22.96	0.73
- Rosecliffe Te	er.	3	Ex. SAN-1	0.000			4.415	0.00		11	0	106	4.2363	0.44	1.32	1.76	0.013	74.2	200	0.57	24.77	0.79
	Ex.	SAN-1	Ex. SAN-2	0.00			4.42	0.00		0	0	106	4.2363	0.44	1.32	1.76	0.013	52.4	250	3.63	113.32	2.31
**	Ex.	SAN-1	Ex. SAN-2	0.00			4.12	0.00		0	273	273	4.0956	0.41	3.27	3.69	0.013	69.2	250	3.63	113.32	2.31

According to City as-built drawing No. 9993, Sanitary Drainage Areas, Rosecliffe Garden Estate, Feb, 1988. The existing 250mm diameter sanitary sewer on Rosecliffe Terrace had been designed with the external area of 4.12 hectares and populations of 273 people.



RESIDENTIAL POPULATION DENSITIES (B)-LOT BASIS (A) AREA BASIS

82 PERSONS PER NET HECTARE

60 PERSONS PER GROSS HECTARE SINGLE FAMILY

SANITARY SEWER DESIGN SHEET

CITY OF LONDON

PROJECT NAME COMMISSIONERS RD. W. LATERAL SANITARY SEWER

CITY ENGINEER'S DEPARTMENT

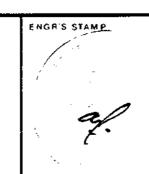
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80m. W. ROSECLIFFE TER. TO 130m. E. ROSECLIFFE TER. LOCATION (HECTARES) SEWAGE FLOW AREA POPULATION SEWER DESIGN PROFILE Δ TOTAL PER No. OF PER | Δ TOTAL INFILT SEWAGE TOTAL SIZE SLOPE FROM TO NETOR MANHOLE MANHOLE GROSS VELOCITY CAP LOSSES FALL IN LENGTH INVERT ELEV. AREA DIMENSIONS STREET ha LOTS LOT POP POP 1/s 1/s IN M.H. SEWER metres I/s No. 1/s m m 16 0.171 0.312 0.483 200 0.50 0.013 0.738 23.97 0,317 62,80 277.110 276.793 AT COMM. RD. W. 1.46 | 1.46 | 60 | 4 | 4 | 16 GROSS 16 0.193 0.312 0.505 200 0.44 0.013 0.692 22.44 0.018 0.279 63.80 276.775 276.496 A2 COMM. RD. W. GROSS 3 60 6 4 24 24 0.335 0.468 0.803 200 0.50 0.013 0.739 23.19 GROSS A3 COMM. RD. W. POSS. FUTURE 1.02 3.88 60 4 4 16 40 0.4540.782 1.236 200 0.49 0.013 0.731 23.70 0.340 69.60 276.690 276.350 A4 COMM. RD. W. GROSS 3 56 0.454 1.096 1.550 250 0.57 0.013 0.915 46.33 0.028 0.032 5.60 276.322 276.290 ROSECLIFFE TER. EX.MH

AS CONSTRUCTED NOTES	AS CONSTRUCTED SERVICES	COMPLETION		No	REVISIONS	DATE	₿Y	CONSULTANT OR DIVISION
	SANITARY SEWER, M.H'S AND		DESIGN L.K	Ī	"AS CONSTRUCTED" DRAWING	OCT./89	J.J.S.	
FURTHER DETAIL	P.D.C'S		DRAWN E.J.H	<u></u>		<u> </u>		
2 SEWER DESIGN TRANSITION WIDTH OR AS NOTED	CUT RESTORATION AS NOTED		CHECKED L.K.	ļ			<u> </u>	
3 REFERENCE B. M. No 5-80	UNDER ROAD RESTORATIONS ON		APPROVED A.W.F					
ELEVATION 234.400 m	THIS DRAWING	MAY/1988	DATE SEPT. 17, 1987		15 A 1 A 1 A 1 A 1 A 1 A 1 A 1 A 1 A 1 A			
ON SOUTH EAST CORNER OF THE CHAMBER OF THE							ļ	
PUMPING STATION ON THE WEST SIDE OF WONDERLAN	D			<u> </u>		ļ		
ROAD AT THE RIVER THAMES.		I		İ			<u> </u>	

- 4 PEOPLE

DUPLEX - 8 PEOPLE
MULTI-FAMILY - BACHELOR - 1½ PEOPLE
-1 BEDROOM - 2½ PEOPLE
-2 BEDROOM - 3½ PEOPLE

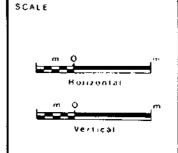


PROJECT ENGINEER





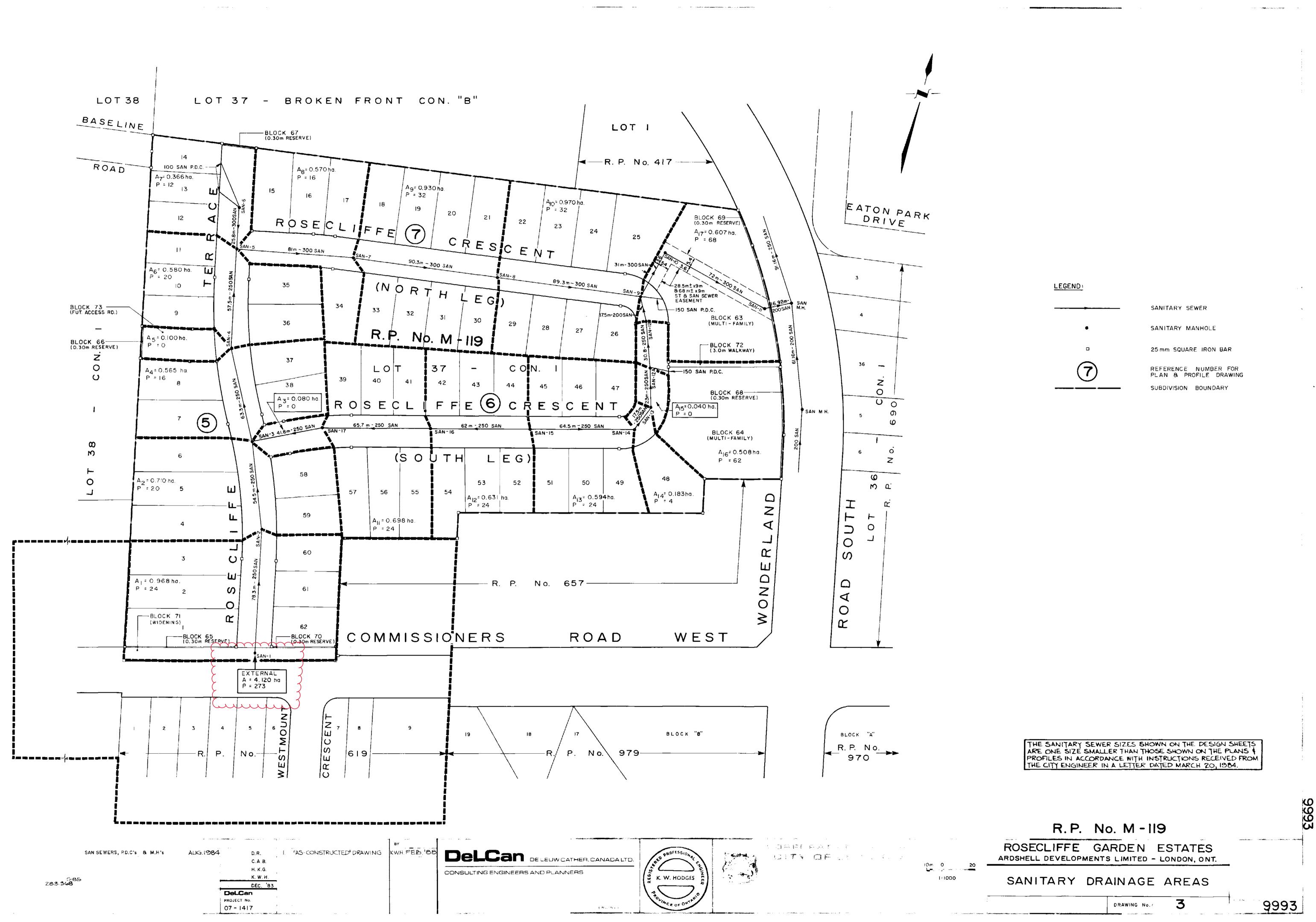
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COMMISSIONERS LATERAL SANIT	· · · · · · · · · · · · · · · · · · ·
SANITARY SEWER	DESIGN SHEET
DRAINAGE	AREAS PLAN

87-610/C/64 10,589

PROJECT NO



Residential	Townhouse	Deve	lonment
Nesidelitiai	10 WHIIIOU3C		IODITICIT

APPENDIX C Stormwater Calculations

PRE-DEVELOPMENT ALLOWABLE FLOWS

2-Year Pre-development Flow

Q= 2.78CIA

C= 0.50 I= 55.602267 mm/h A= 0.445 ha Pre-development Run-off Coefficient Rainfall intensity, Time to Peak = 19 minutes Lot drainage area

Q = 34.4 l/sec

100-Year Pre-development Flow

Q= 2.78CIA

I= 131.48404 mm/h Rainfall intensity, Time to Peak = 19 minutes

Q = 81.3 l/sec

POST-DEVELOPMENT - Site STORAGE REQUIREMENT

Design Criteria:

 $\begin{array}{cccc} & Lot \ Area & A= & 0.399 \ ha \\ Post-Development & C_{2-yr}= & 0.65 \\ Post-Development & C_{100-year}= & 0.65 \\ \hline Flow & Q= & 2.78CIA \ m^3 \end{array}$

Storm	2 Year
а	754.36
b	6.01
С	0.81

Q_{pre_2} = 34.4 l/s

Duration	Intensity	Peak Runoff	Storm runoff	Release Flow	Req'd Storage
(minute)	mm/hr	m³/s	m ³	m ³	m ³
5	108.1	0.07788	23.36	10.32	13.05
6	100.7	0.07258	26.13	12.38	13.75
7	94.4	0.06803	28.57	14.44	14.13
8	88.9	0.06407	30.75	16.51	14.24
9	84.1	0.06059	32.72	18.57	14.15
10	79.8	0.05751	34.50	20.64	13.87
13	69.4	0.05004	39.03	26.83	12.20
15	64.0	0.04614	41.53	30.95	10.58
19	55.6	0.04007	45.68	39.21	6.47
20	53.9	0.03882	46.58	41.27	5.31
21	52.2	0.03765	47.44	43.33	4.10
22	50.7	0.03656	48.25	45.40	2.86
23	49.3	0.03553	49.03	47.46	1.57
24	48.0	0.03457	49.78	49.53	0.25
25	46.7	0.03366	50.50	51.59	-1.09

Storm	100 Year
а	2619.36
b	10.50
С	0.88

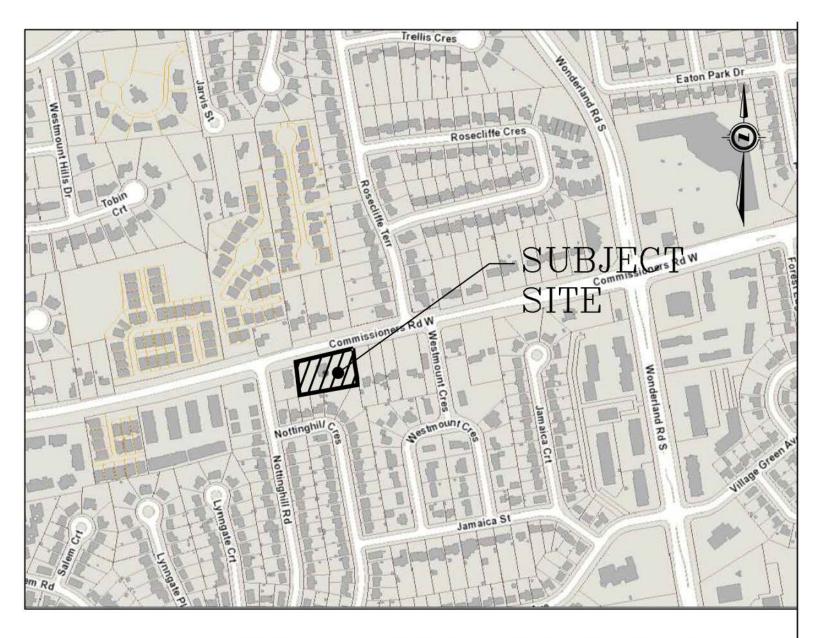
 $Q_{pre_100} = 81.3 \text{ I/s}$

O	0.00				
Duration	Intensity	Peak Runoff	Storm runoff	Release Flow	Req'd Storage
(minute)	mm/hr	m³/s	m ³	m ³	m ³
5	232.2	0.167	50.21	24.40	25.81
6	219.8	0.158	57.01	29.28	27.73
7	208.6	0.150	63.14	34.16	28.98
8	198.6	0.143	68.70	39.04	29.66
9	189.6	0.137	73.78	43.92	29.86
10	181.4	0.131	78.43	48.80	29.63
11	173.9	0.125	82.71	53.68	29.04
13	160.8	0.116	90.36	63.44	26.92
15	149.6	0.108	97.00	73.20	23.80
19	131.5	0.095	108.02	92.72	15.30
20	127.7	0.092	110.40	97.60	12.80
21	124.1	0.089	112.66	102.48	10.19
22	120.7	0.087	114.81	107.35	7.45
23	117.5	0.085	116.86	112.23	4.62
24	114.5	0.083	118.81	117.11	1.69
25	111.6	0.080	120.67	121.99	-1.32

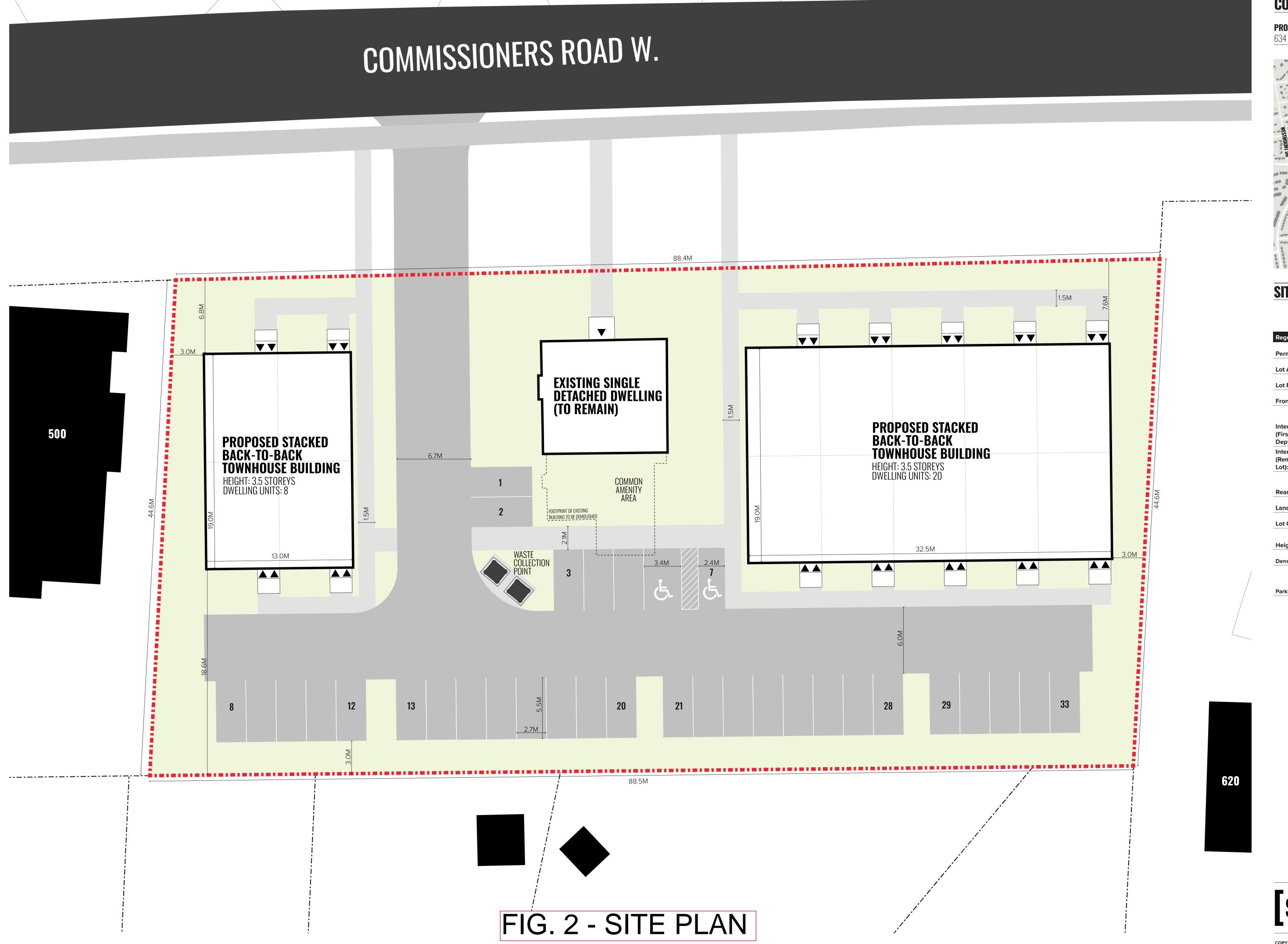
Residentia	l Town	house I	Devel	lopment
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APPENDIX D

<u>Figures</u>



SK 1. SITE LOCATION



Lot Boundary Disclaimer: Site dimensions have been assumed based on data provided by the City of London. Siv-ik planning and design inc. makes no warranties or guarantees regarding the accuracy of the lot boundaries.

CONCEPT PLAN

PROJECT SITE



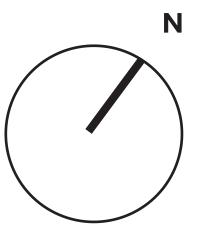
634 Commissioners Road W



SITE DATA

		LUIM
Regulations	Required	Proposed
Permitted Uses:	Section 9.2	Cluster Stacked Townhouse Dwellings
Lot Area:	1,000m² (min.)	4,499.3m²
Lot Frontage:	30.0m (min.)	88.4m
Front Yard:	6.5m (min.)	6.8m*
Interior Side Yard (First 30m of Lot Depth):	1.8 metres (5.9 feet) when the end wall of a unit contains no windows to habitable rooms, or 6.0 metres (9.8 feet) when the wall of a unit contains windows to habitable rooms.	3.0m
Interior Side Yard (Remainder of Lot):	3.0m (min.)	3.0m
Rear Yard:	1.0 metre per 1.0 metre of main building height, but in no case less than 6.0 metres.	18.6m
Landscape OS:	30% (min.)	38.4%
Lot Coverage:	45% (max.)	21.5%
Height:	12.0m (max.)	12.0m
Density:	25uph (max.)	65uph*
	Stacked Townhouse: 0.5/unit	
	Single Detached: 1 per unit	
Parking:	18 total required	33 total provided
		* Requires Special Provision

[09.27.23] Plan Scale: 634CW 2.0



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